

SUPERVISOR'S DECLARATION

I hereby declare that I have checked this thesis and in my opinion, this thesis is adequate in terms of scope and quality for the award of the degree of Bachelor of Engineering (Hons) Civil Engineering.

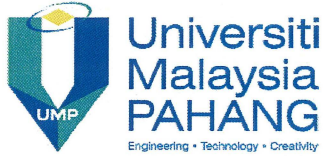


(Supervisor's Signature)

Full Name : YOUVENTHARAN DURAISAMY, PhD

Position : LECTURER

Date : 14 JUNE 2016



STUDENT'S DECLARATION

I hereby declare that the work in this thesis entitled "Influencing Factor of Crushable Sand Soil on Liquefaction" is based on my original work except for quotations and citations which have been duly acknowledged. I also declare that it has not been previously or concurrently submitted for any other degree at Universiti Malaysia Pahang or any other institutions.

A handwritten signature in black ink, appearing to read 'Ths.', is positioned above a horizontal line.

(Student's Signature)

Full Name : THARUSHINI A/P THIAGARAJAN

ID Number : AA13266

Date : 14 JUNE 2017

INFLUENCING FACTOR OF CRUSHABLE SAND SOIL
ON LIQUEFACTION

THARUSHINI A/P THIAGARAJAN

Thesis submitted in fulfillment of the requirements
for the award of the
Bachelor Degree in Civil Engineering

Faculty of Civil Engineering and Earth Resources
UNIVERSITI MALAYSIA PAHANG

JUNE 2017

PERPUSTAKAAN UNIVERSITI MALAYSIA PAHANG	
No. Perolehan 120967	No. Panggilan FKASA
Tarikh 23 NOV 2017	T43 2017 r BC

ACKNOWLEDGEMENTS

I would like to take this opportunity to thank all the parties who helped me in completing this research. First of all, I would like to express my sincere appreciation to my research supervisor, Dr Youventharan Duraisamy for his guidance and support during planning and development of this research work. His personal kindness, skill and patience are highly appreciated. Without his continuous support and interest, this final year project would have not been presented here.

Besides that, I would like to acknowledge Universiti Malaysia Pahang for giving me this opportunity to conduct this research and provided me with a comfortable working environment and refined equipment. I would like to extend my thanks to the technicians of Soil Mechanics & Geotechnical Laboratory for their assistance in my laboratory work and for their help in offering me the resources in running the laboratory tests.

Last but not least, I would like to express my heartfelt gratitude to my parents and family. Thanks for their sacrifice, continuous support and encouragement throughout the completion of my final year project. A special thanks also goes to all my dearest friends for their kindness, helps and supports.

ABSTRAK

Rintangan pencairan pasir sangat dipengaruhi oleh ciri-ciri tanah itu sendiri. Penghancuran atau pecahan butiran pasir berlaku apabila terdapat tekanan yang berlebihan dikenakan pada tanah pasir dan terdapat pengurangan dalam tekanan berkesan pada tanah dan peningkatan tekanan air liang yang menyebabkan pencairan. Dalam kajian ini, tiga lokasi di Pantai Timur Semenanjung Malaysia telah dipilih di mana aktiviti-aktiviti penghancuran tanah seperti pemasangan cerucuk yang biasanya berlaku. Oleh itu, kajian ini dijalankan untuk mengenalpasti faktor utama yang mempengaruhi keretakan tanah untuk berlaku pencairan. Faktor yang mempengaruhi yang telah dikaji dalam kajian ini ialah ketumpatan nisbi dan kesegian. Sampel tanah pasir dari kawasan yang berbeza telah dikumpulkan. Sampel tanah telah dihancurkan dengan menggunakan pemadat automatik untuk 500 dan 1000 pukulan. Selepas proses penghancuran, sampel pasir sebelum dan selepas dihancurkan telah terlibat dalam satu siri ujian di makmal tanah dan geoteknik iaitu analisa ayakan, kaedah piknometer untuk ujian analisa gravity tentu dan uji kaji ketumpatan nisbi untuk menentukan sifat-sifat tanah seperti gravity tentu, ketumpatan nisbi dan pecahan butiran tanah. Mikroskop digital USB telah digunakan untuk menentu butiran tanah untuk mengira indeks kesegian bagi tanah pasir sebelum dan selepas dihancurkan. Penghancuran butiran pasir selepas 500 dan 1000 pukulan dianalisis dengan menggunakan D_{50} Index Pemecahan. Ketumpatan nisbi dan kesegian sampel pasir sebelum dan selepas dihancurkan dikaji mengikut D_{50} Index Pemecahan untuk mengetahui hubungan antara faktor mempengaruhi penghancuran tanah dan potensi pencairan. Berdasarkan keputusan yang diperolehi, ia adalah jelas ketara bahawa kesegian tanah pasir yang dihancur amat mempengaruhi pencairan untuk berlaku.

ABSTRACT

Liquefaction resistance of sand is highly influenced by the characteristics of soil itself. Sand particle crushing or breakage occur when there is excessive stress exert on the sand soil and lead to reduction in effective stress of soil and increase in pore water pressure that result in liquefaction. In this research, the three locations in East Coast Peninsular Malaysia were chosen where the activities of crushing such as pile installation commonly occurs. When the liquefaction potential of the sand soil being affected, this will lead to the failure of construction design which was designed based on existing sand properties by neglecting the conditions of the sand after being crushed. Hence, this research was conducted to identify the factor that highly influence the soil's susceptibility to liquefaction. The influencing factors that have been studied in this research are relative density and angularity. The sand soil samples from different area were collected. The samples were crushed by using automatic compactor for 500 and 1000 blows. After the crushing process, the sand samples before and after crushed were subjected to a series of test in the soil and geotechnics laboratory which are sieve analysis test, pycnometer method for specific gravity test and relative density test in order to determine the properties of soil such as, specific gravity, relative density and particle size distribution. The USB digital microscope was used to capture the soil particles in order to calculate the angularity index of the sand soil before and after crushed. The crushability of sand particle after 500 and 1000 blows was analyzed by using D_{50} Breakage Index. The relative density and angularity of sand samples before and after crushed were studied according to the D_{50} Breakage Index in order to find out the relationship between influencing factor of crushable soil and liquefaction potential. Based on the result obtained, it is clearly significant that angularity of crushable sand is highly influencing liquefaction.

TABLE OF CONTENTS

DECLARATION

TITLE PAGE

ACKNOWLEDGEMENTS **ii**

ABSTRAK **iii**

ABSTRACT **iv**

TABLE OF CONTENTS **v**

LIST OF TABLES **viii**

LIST OF FIGURES **ix**

LIST OF SYMBOLS **xi**

LIST OF ABBREVIATIONS **xiii**

CHAPTER 1 INTRODUCTION **1**

1.1 Introduction and Background 1

1.2 Problem Statement 2

1.3 Research Question 3

1.4 Research Objectives 3

1.5 Scope and Limitation 4

CHAPTER 2 LITERATURE REVIEW **5**

2.1 Overview of Chapter 5

2.2 Liquefaction of soil 5

2.3 Sand Particle Breakage 10

2.4 Influencing Factors of Sand Soil 14

2.4.1	Angularity of Sand Soil	14
2.4.2	Relative Density of Sand Soil	15
2.5	Summary	17
CHAPTER 3 METHODOLOGY		18
3.1	Introduction	18
3.2	Sampling Location	18
3.3	Sampling Work	19
3.4	Material Preparation	20
3.5	Laboratory Testing	21
3.5.1	Sieve Analysis	21
3.5.2	Moisture Content Test	22
3.5.3	Specific Gravity test	23
3.5.4	Relative Density Test	25
3.6	Angularity of Sand	28
3.7	Breakage Index	29
CHAPTER 4 RESULTS AND DISCUSSIONS		30
4.1	Overview of Chapter	30
4.2	Crushing of Sand Particle	30
4.3	Relationship between Relative Density of Crushable Sand and Liquefaction	34
4.4	Relationship between Angularity of Crushable Sand and Liquefaction	39
4.5	Most Influencing factor of Crushable Sand Soil on Liquefaction	44
CHAPTER 5 CONCLUSION AND RECOMMENDATIONS		45
5.1	Introduction	45

5.2	Conclusion	45
5.3	Recommendations	46
REFERENCES		48
APPENDICES		52
Appendix A		52
Appendix B		57
Appendix C		59

LIST OF TABLES

Table 2.1	Classification of Genus Sediments parameter	10
Table 4.1	Progression of Cu, Cc and the classification of the soil samples	32
Table 4.2	Specific gravity and relative density of soil samples	34
Table 4.3	Liquefaction zone based on relative density for soil samples	38
Table 4.4	Liquefaction zone based on angularity factor for soil samples	36
Table 4.5	Microphotograph of soil samples	43
Table 4.6	Liquefaction zone based on relative density and angularity factor	36
Table A1	Sieve Analysis of Teluk Cempedak Soil Sample (Before Crushing)	52
Table A2	Sieve Analysis of Teluk Cempedak Soil Sample (Crushed-500 Blows)	52
Table A3	Sieve Analysis of Teluk Cempedak Soil Sample (Crushed-1000 Blows)	53
Table A4	Sieve Analysis of Taman Gelora Soil Sample (Before Crushing)	53
Table A5	Sieve Analysis of Taman Gelora Soil Sample (Crushed-500 Blows)	54
Table A6	Sieve Analysis of Taman Gelora Soil Sample (Crushed-1000 Blows)	54
Table A7	Sieve Analysis of Pantai Batu Hitam Soil Sample (Before Crushing)	55
Table A8	Sieve Analysis of Pantai Batu Hitam Soil Sample (Crushed-500 Blows)	55
Table A9	Sieve Analysis of Pantai Batu Hitam Soil Sample (Crushed-1000 Blows)	56
Table B1	Specific Gravity of Soil Sample (Before Crushing)	57
Table B2	Specific Gravity of Soil Sample (Crushed-500 Blows)	58
Table B3	Specific Gravity of Soil Sample (Crushed-1000 Blows)	58
Table C1	Relative Density of Soil Sample (Before Crushing)	59
Table C2	Relative Density of Soil Sample (Crushed-500 Blows)	60
Table C3	Relative Density of Soil Sample (Crushed-1000 Blows)	61

LIST OF FIGURES

Figure 2.1	Grain size distribution curves for liquefaction-susceptible soils and Sabarmati-river and Toyoura sands	7
Figure 2.2	Gradation curves for the samples collected from liquefied areas	7
Figure 2.3	Interpretation of curve with magnitude and epicentral distance	8
Figure 2.4	Liquefaction parameter (T) versus probability of liquefaction (P) for different time exposures at Surabaya east coastal plain zone	9
Figure 2.5	Visualisation of fracture process of LBS-1 from three scans	12
Figure 2.6	The effect of relative density on the relationship between cyclic stress ratio and number of cycles	16
Figure 2.7	Relative Density versus grain size relationship for accelerations of 0.3g and 0.6g	16
Figure 3.1	Soil Sample Location	19
Figure 3.2	Automatic Compactor	20
Figure 3.3	Hammer Drop	20
Figure 3.4	Mechanical Sieve Shaker	21
Figure 3.5	Weighing the sample after sieving	21
Figure 3.6	Weighing the mass of container and sand sample	22
Figure 3.7	Bottles with soil suspension that kept in vacuum desiccators	24
Figure 3.8	Bottles with soil suspension that kept in room temperature	24
Figure 3.9	Pouring sand through funnel	27
Figure 3.10	Trimming soil surface by using straightedge	27
Figure 3.11	The mold filled with soil attached with surcharge weight on vibrating table	27
Figure 3.12	The dial indicator gauge readings were recorded	27
Figure 3.13	USB Digital Microscope	28
Figure 4.1	Particle size distribution of Teluk Cempedak sand soil	31
Figure 4.2	Particle size distribution of Taman Gelora sand soil	31
Figure 4.3	Particle size distribution of Pantai Batu Hitam sand soil	32

Figure 4.4	D ₅₀ Breakage Index	33
Figure 4.5	Relative Density versus D ₅₀ Breakage Index	35
Figure 4.6	Void Ratio versus Relative Density	35
Figure 4.7	Void Ratio versus Relative Density (Boundary when Dr=40%)	37
Figure 4.8	Classification of Liquefaction Zone based on relative density factor	37
Figure 4.9	Angularity Index versus Breakage Index	39
Figure 4.10	Void Ratio versus Angularity Index	40
Figure 4.11	Void Ratio versus Angularity Index (Boundary when AI=0.22)	41
Figure 4.12	Classification of Liquefaction Zone based on angularity factor	41

LIST OF SYMBOLS

%	Percent
km	Kilometre
mm	Millimetre
μm	Micrometre
kg	Kilogram
g	Gram
kJ/m^3	Kilojoule per cubic metre
$^{\circ}\text{C}$	Degree Celsius
ρ	Density
ρ_{dmin}	Minimum dry density
ρ_{dmax}	Maximum dry density
e	Void ratio
e_{min}	Minimum void ratio
e_{max}	Maximum void ratio
M_b	Body-wave magnitude
M_w	Moment magnitude
NL	Number of Layers
NB	Number of Blows
W	Weight of hammer
D	Distance between hammer & sample
V_m	Volume of mold
M_{cws}	Mass of container and wet sand soil sample
M_{cds}	Mass of container and oven-dried sand soil sample
M_c	Mass of container
W_1	Weight of bottle and stopper
W_2	Weight of bottle, stopper and dry soil
W_3	Weight of bottle, stopper, soil and water
W_4	Weight of bottle, stopper and water
M_{S1}	Mass of tested-dry soil 1
M_{S2}	Mass of tested-dry soil 2
V_c	Calibrated Volume of the mold
V	Volume of tested-dry soil
D_r	Relative Density
$R_{p\theta}$	The radius of the particle at a directional angle θ

$R_{EE\theta}$	The radius of an equivalent ellipse at a directional angle θ
B_g	Breakage Index
SP	Poorly Graded Sand
C_u	Uniformity coefficient
C_c	Coefficient of curvature
D_{10}	10% pass particle size in diameter
D_{30}	30% pass particle size in diameter
D_{50}	50% pass particle size in diameter
D_{60}	60% pass particle size in diameter
R^2	Coefficient of Determination
HL	Highly Liquefiable
ML	Moderately Liquefiable
LL	Low Liquefiable
NL	Non Liquefiable

LIST OF ABBREVIATIONS

M_b	Body-wave magnitude
M_w	Moment magnitude
USB	Universal Serial Bus
FS	Factor of Safety
LPI	Liquefaction Potential Index
ASTM	American Society for Testing and Materials
AI	Angularity Index
USCS	Unified Soil Classification System

CHAPTER 1

INTRODUCTION

1.1 Introduction and Background

Liquefaction is the phenomena that have been widely observed which occurs in saturated granular loose sands during numerous traumatic earthquake. Several historical earthquakes like in 2011 Christchurch earthquake in New Zealand, in 2015 Gorkha earthquake in Nepal, in 2016 Kumamoto earthquake in Japan, in 2017 Surigao Del Norte earthquake in Philippine have delineated the disastrous effect of soil liquefaction on buildings and lifelines. (Gautam, Magistris, & Fabbrocino, 2017; Goda, 2016; Serrano, 2017) According to previous research, more than 130 earthquake phenomena had occurred in Sabah with magnitude ranging from 1.9 to 5.9 Mb. Recently in 2015, Sabah has experienced a destructive earthquakes at Kinabalu with magnitude of 5.9 Mb at 10km depth. (Lim, 2015; Chan, 2017). Moreover, Ranau and Kunak have been experienced a mild earthquake with magnitude 4.2 Mb and 3.2 Mb lately on March 2017 (Inus, 2017). Therefore, it is wise to be aware of the liquefaction potential in Peninsular Malaysia as Sabah is proven to be vulnerable to liquefaction.

Crushable sand which caused by high stress exerted on the sand particles are considered fragile and are frequently encountered in places where liquefaction phenomena occurred (Liu, 2015). Crushed sand soils are normally resulted from human activities such as pile installation, construction of high earth or rock fill dams, impact of projectiles, laying foundations of the offshore gravity structures and so on (Lobo-Guerrero and Vallejo, 2005; Wood, 2006; Bartake and Singh, 2007). The characteristic of the crushable sand soils will change the original engineering properties which a structure was initially designed accordingly. This influence the possibility of soil

liquefaction to be occurred under consideration of the effect from the new soil properties after being crushed.

From the past history of liquefaction, it has illustrated that there are few factors that can be considered with the liquefaction susceptibility of granular soils. Past researchers have indicated that liquefaction susceptibility of sand soil decreases when the relative density increases. There is a limit value where the sand particles become susceptible to liquefaction in term of grain size of sand soils (Hakam, 2016). In respect of angularity of sand soils, the more angular the sand is the more susceptible the sand is to liquefaction compared to rounded sand at high confining pressure even at the relative density approaching 100% for moderate earthquakes (Chern, Tumi, & Student, 1986). Only few studies in the literature have specifically evaluated any of the factors such as angularity, particle size distribution or relative density alone that influence the occurrence of liquefaction. However in an attempt to relate the relative density and angularity, this research have been carried out to analyse the most critical influencing factors of sand soil on liquefaction.

1.2 Problem Statement

Sand crushing will commonly occur in construction site. For example, sand particle breakage happen during pile driving, compaction for cut and fill, compaction during road and earthwork construction. The sand crushing or breakage of sand soil particle due to exposure of high pressure on the sand affects the properties of the existing sand soil. This can lead to occurrence of liquefaction after the crushing of sand particles took place when the crushed sand does not maintain the same properties which they had during design stage. There were influencing factor of crushable sand particle that lead to the possibility of soil susceptible to liquefaction which are relative density and angularity. Hence, a research was conducted to identify the influencing factor of crushable sand soil that lead to crushing and analyse the most critical influencing factor of crushable sand soil on liquefaction.

1.3 Research Question

This study aimed to address the following research questions.

- i. Does the denser soil liquefy easily when the soil is crushable?
- ii. Does the more angular soil liquefy easily when the soil is crushable?
- iii. Does density of sand is the most influencing factor for liquefaction than the angularity of the sand?

1.4 Research Objectives

The main purpose of this research is to study the influencing factor of crushable sand soil from East Coast Peninsular Malaysia on liquefaction susceptibility. Three specific objectives have been listed below in order to achieve the aim of this research.

- i. To determine the relationship between soil relative density and crushability.
- ii. To determine the relationship between soil angularity and crushability.
- iii. To identify the most influencing factor of crushable sand soil on liquefaction.

REFERENCES

- Agung, P. A. M., & Ahmad, M. A. (2014). Potential Liquefaction Of Loose Sand Lenses : Case Study in Surabaya East Coastal Plain , Indonesia, *International Journal of Integrated Engineering*, 6(2), 1–10.
- Bartake, P.P. and Singh, D.N. (2007). A Generalized Methodology For Determination Of Crushing Strength Of Granular Materials, *Geotechnical Geology Engineering*, 25, 203–213.
- Borowiec.A, & Maciejewski.K. (2014). Assessment Of Susceptibility To Liquefaction Of Saturated Road Embankment Subjected To Dynamic Loads, *Studia Geotechnica et Mechanica*, XXXVI(1).
- Chan, J. (2017, January 10). Sabah rattled by 7.3 quake in nearby Philippines, *Malay mail online*.
- Chern, J. C., Tumi, H., & Student, D. (1986). Confining Pressure, Grain Angularity and Liquefaction, *Journal of Geotechnical Engineering*, I(10), 1229–1235.
- Chuhan, F.A., Kjeldstad A., Bjorlykke, K. and Hoeg, K. (2003). Experimental compression of loose sands: relevance to porosity reduction during burial in sedimentary basins, *Canadian Geotechnical Journal*, 40, 995–1011.
- Coop, M.R., and Lee, I.K. (1993). The behavior of granular soils at high stresses, *In Predictive soil mechanics*. Edited by G.T. Housbly and A.N. Schofield. Thomas Telford, London, U.K., 186–198.
- Dinesh, S. V, Kumar, G. M., Balreddy, M. S., & Swamy, B. C. (2011). Liquefaction Potential Of Sabarmati-River Sand, *Journal of Earthquake Technology*, 48(516), 61–71.
- Feda, J. (2002). Notes on the effect of grain crushing on the granular soil behaviour, *Engineering Geology*, 63(1-2), 93-98.
- Forootan, M., Silakhori, E., & Alvandi, E. (2015). Soil Liquefaction Hazard Zonation Map for Kordkuy County , Golsetan Province Using Model SWM, *Iranian Journal of Earth Science*, 7, 2013–2016.
- Gautam, D., Magistris, F. S. De, & Fabbrocino, G. (2017). Soil liquefaction in Kathmandu valley due to 25 April 2015 Gorkha, Nepal earthquake. *Soil Dynamics and Earthquake Engineering*, 97(April 2015), 37–47.
- Ghafghazi, M., Shuttle, D. A., & Dejong, J. T. (2014). Particle breakage and the critical state of sand. *Soils and Foundations*, 54(3), 451–461.

Goda, K. (2016). The 2016 Kumamoto earthquakes: cascading geological hazards and compounding risks, *Frontiers in Built Environment*, 2(August), 1–23.

Hagerty, B. M. M., Member, A., Hite, D. R., Ullrich, C. R., & Hagerty, D. J. (1993). One-Dimensional High-Pressure Compression, *Journal of Geotechnical Engineering*, 119(1), 1–18.

Hagerty, M.M., Hite, D.R., Ulrich, C.R., Hagerty, D.J. (1993). One dimensional high pressure compression of granular media, *ASCE Journal of Geotechnical Engineering*, 119(1), 1–18.

Hakam, A. (2016). Laboratory Liquefaction Test of Sand Based on Grain Size and Relative Density, *Journal of Engineering and Technological Sciences*, 48(3), 334–344.

Hardin, B.O. (1985). Crushing of soil particles, *ASCE Journal of Geotechnical Engineering*, 11(10), 1177–1192.

Hendron A.J. (1963). The behavior of sand in one-dimensional compression, *PhD thesis, University of Illinois at Urbana-Champaign, Illinois, USA*.

Hoff, I., Nordal, S. and Nordal, R.S. (1999). Constitutive Model for Unbound Granular Materials Based on Hyperelasticity, *Workshop on Modelling and Advanced Testing for Unbound Granular Materials, Lisbon, Portugal*.

Huang, C. (2015). Study on Particle Breakage of Sands Subjected to Various Confining Stress and Shear Strain Levels, Tokyo Metropolitan University.

Inus, K. (2017, March 26). After Ranau, small quake hits Kunak, *New Straits Times*.

Iwasaki, T., Tokida, K., Tatsuko, F., and Yasuda, S. (1978). A practical method for assessing soil liquefaction potential based on case studies at various sites in Japan, *Proceedings of 2nd International Conference on Microzonation, San Francisco*, 885–896.

Kong, H. (2015). An investigation of single sand particle fracture using X-ray micro-tomography, *Geotechnique* (8), 625–641.

Kramer, S.L. (1996). Geotechnical Earthquake Engineering, *Prentice Hall, New Jersey*.

Ku, C.S. and Chi, Y.Y. (2006). A study of the CPT-based liquefaction potential index, *Geophysical Research Abstracts*, 8, 01292.

Lade, B. P. V., Yamamuro, J. A., & Bopp, P. A. (1996). Significance Of Particle Crushing In Granular Materials By Poul V. Lade, Jerry A. Yamamuro, and Paul A. Bopp3, *Journal of Geotechnical Engineering*, 122(April), 309–316.

Lee, K.L and Farhoomand, I. (1967). Compressibility and crushing of granular soil in anisotropic triaxial compression, *Canadian Geotechnical Journal* 4(1), 68–86.

Lenz, J.A. and Baise, L.G. (2007). Spatial variability of liquefaction potential in regional mapping using CPT and SPT data, *Soil Dynamics and Earthquake Engineering*, 27, 690–702.

Lim, C.H. (2015, June 23). Engineering group ready to help, *The Star*.

Little, D., Button, J., & Jayawickrama, P. (2003). Quantify Shape, Angularity and Surface Texture of Aggregates Using Image Analysis and Study Their Effect on Performance, Texas Transportation Institute, 7(2).

Liu, L. (2015). Crushing-induced liquefaction characteristics of pumice sand, *New Zealand Society for Earthquake Engineering (NZSEE) Annual Technical Conference*.

Lobo-Guerrero, S., Vallejo, L.E. (2005). Crushing a weak granular material: experimental numerical analyses, *Geotechnique*, 55(3), 245–249.

Marsal, R.J. (1967). Large-scale testing of rockfill materials, *Journal of the Soil Mechanics and Foundations Division*, 93(2), 27–43.

Miura, N., Murata, H. and Yasufuku, N. (1984). Stress-strain characteristics of sand in a particle-crushing region, *Soils and foundations*, 24(1), 77–89.

Papathanassiou, G., Pavlides, S. and Ganas, A. (2005). The 2003 Lefkada earthquake: field observation and preliminary microzonation map based on liquefaction potential index for the town of Lefkada, *Engineering Geology*, 82, 12–31.

Poulos, S.J., Castro, G. and France, J.W. (1985). Liquefaction evaluation procedure, *ASCE Journal of Geotechnical Engineering*, 111(6), 772–792.

Seed, H.B. and Idriss, I.M. (1971). Simplified procedure for evaluating soil liquefaction potential, *Journal of the Soil Mechanics and Foundation Division*, 97, 1249–1273.

Serrano, B. (2017, February 12). Surigao quake kills 6, *The Philippine Star*.

Sonmez, H. and Gokceoglu, C. (2005). A liquefaction severity index suggested for engineering practice, *ASCE Environmental Engineering*, 129(4), 315–322.

Sonmez, H. (2003). Modification of the liquefaction potential index and liquefaction susceptibility mapping for a liquefaction-prone area (Inegol, Turkey), *Environmental Geology*, 44, 862–871.

- Tuttle, M., Chester, J., Lafferty, R., Dyer-Williams, K., and Cande, B. (1999). Paleoseismology study northwest of the New Madrid Seismic Zone, *US Nuclear Regulatory Commission, NUREG/CR-5730, Washington, DC*.
- Ulusay, R. and Kuru, T. (2004). 1998 Adana-Ceyhan (Turkey) earthquake and a preliminary microzonation based on liquefaction potential for Ceyhan Town, *Natural Hazards*, 32, 59–88.
- Wood, D.M. (2006). 20th Bjerrum Lecture: the magic of sands, *Norsk Geoteknisk Forening, Oslo*.
- Yang, Z.X., Jardine, R.J., Zhu, B.T., Foray, P. and Tsuha, C.H.C. (2010). Sand grain crushing and interface shearing during displacement pile installation in sand, *Geotechnique*, 60(6), 469–482.
- Youd, T.L. and Perkins, D.M. (1978). Mapping liquefaction-induced ground failure potential, *Journal of Geotechnical Engineering Division*, 104, 443–446.
- Youd, T.L., Idriss, I.M., Andrus, R.D., Arango, I., Castro, G., Christian, J.T., Dobry, R., Finn, W.D.L., Harder Jr., L. F., Hynes, M.E., Ishihara, K., Koester, J.P., Liao, S.S.C., Marcuson III, W.F., Martin, G.R., Mitchell, J.K., Moriwaki, Y., Power, M.S., Robertson, P.K., Seed, R.B., and Stokoe II, K.H. (2001). Liquefaction resistance of soils summary report from 1996 NCEER and 1998 NCEER/NSF workshops on Evaluation of Liquefaction Resistance of Soil, *Journal of the Geotechnical Geoenvironment Engineering*, 127, 817–833.
- Zheng, J., Wong, T.F., Yanagidani, T. and Davis, D.M. (1990). Pressure induced microcracking and grain crushing in Berea and Boise sandstone-acoustic emission and quantitative measurements, *Mechanics Mater*, 9, 1–5.